



THAYER SCHOOL OF  
ENGINEERING  
AT DARTMOUTH

# STRUCTURAL ANALYSIS OF EARTH-BAG SYSTEMS

ENGS 88 – Honors Thesis

Awais Malik

Faculty Advisor: Professor Vicki V. May

28<sup>th</sup> May 2013

[engineering.dartmouth.edu](http://engineering.dartmouth.edu)

# AGENDA

- § Background
- § Project Goals
- § Design Methodology
- § Construction
- § Shake Table Testing
- § Observations and Results
- § Analysis
- § Development of CAD Models
- § SolidWorks Simulations
- § Conclusions
- § Recommendations for Future Work
- § Acknowledgements



# WHAT IS AN “EARTH-BAG SYSTEM?”

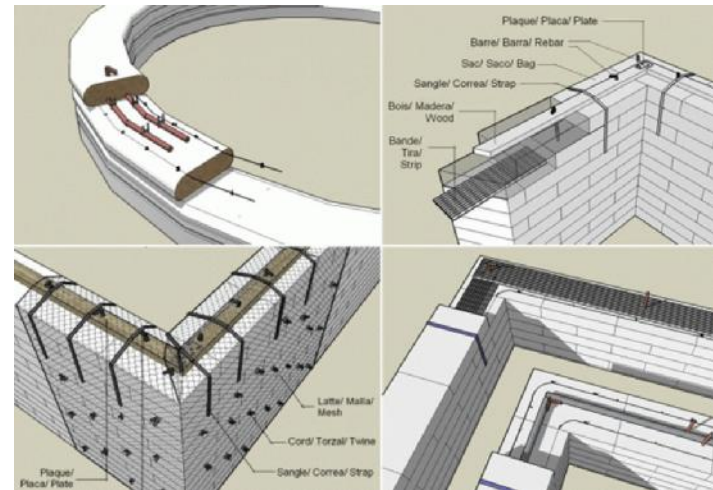
Masonry wall in which the building blocks are earth-bags: sturdy sacks filled with inorganic material (soil).

## Reinforcement Techniques:

- Barbed wire is often placed between the courses to improve friction.
- Rebar and twine are hammered in to provide more resistance against overturning.
- Earth-bag walls are finally plastered.

## Advantages:

- Inexpensive method to create structures that are both strong and can be quickly built.
- Highly insulated walls with excellent performance in humidity.
- Eco-friendly!
- Provides Greater Resistance to **Earthquakes**.  
**(NEED TESTING TO VERIFY THIS CLAIM)**



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# EXAMPLES OF EARTH-BAG SYSTEMS



- Historically, earth-bag walls are used as military bunkers and temporary flood controlling dikes.



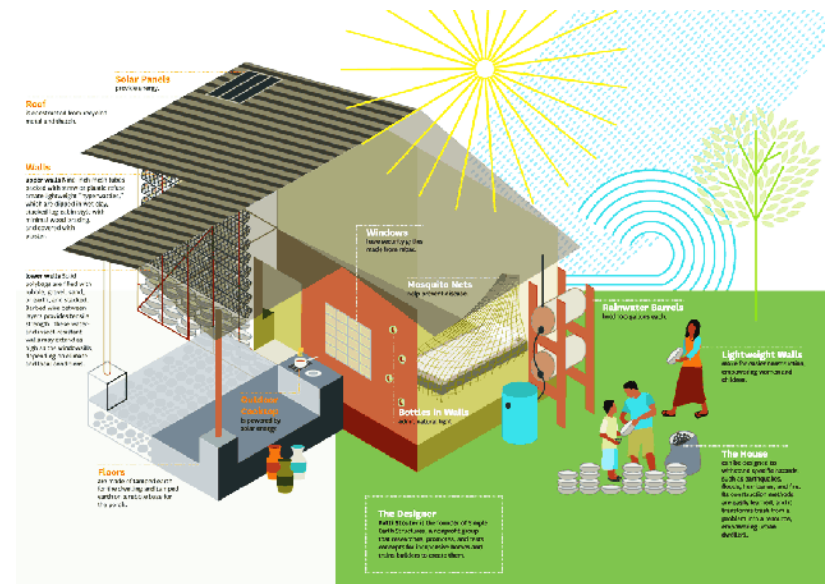
- Earth-bags are also one of the most popular choices for constructing **affordable housing** in developing countries.

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# THE \$300 HOUSE PROJECT

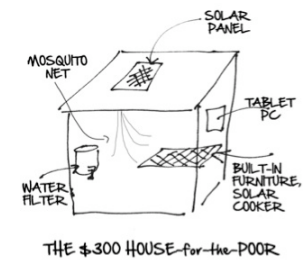
- Dartmouth's initiative to design and implement affordable housing solutions for Haiti.
- Professor Vicki May is part of a team of designers creating innovative ways to **reduce** construction **costs** and make implementation **simpler**.
- Challenge is to ensure **safety** of proposed structures.
- Prototype houses are being built in Haiti. Team led by Professor **Jack Wilson**.



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# THE \$300 HOUSE CHALLENGE

- Blog post in the Harvard Business Review by Professor **Vijay Govindarajan** and Christian Sarkar challenging designers to create a house for the poor, ideally for \$300.  
(<http://www.300house.com>)
- The Design Challenge winners attended a Design Workshop at Dartmouth in Jan 2012.



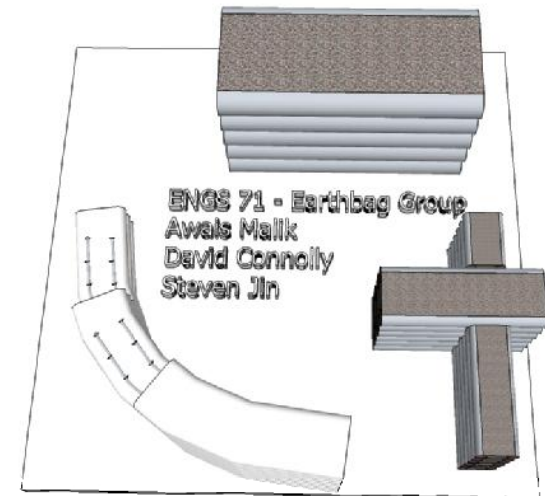
**Patti Stouter**, one of the winners of the \$300 House Challenge, is an expert in Earth-bag construction. She gave an ENGS 71 guest lecture emphasizing the need for experimental testing of alternate materials.



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# ENGS 71 GROUP PROJECT

- Professor May presented the \$300 House Challenge to her students in ENGS 71 (Structural Analysis) in Spring 2012.
- I was a member of the group investigating the structural viability of earth-bags.



STEP 1



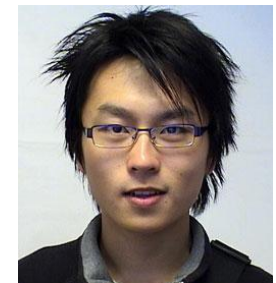
STEP 2



STEP 3



David Connolly  
'13



Steven Jin  
'12

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# ENGS 71 GROUP PROJECT

- Our project goal was to investigate the effectiveness of rebar as an added reinforcement.
- We built two walls scaled down to 40%. One wall had rebar hammered through at regular intervals.
- The test setup included a hydraulic jack connected to a force gage. A ruler was fixed along the jack to measure the displacement.
- We measured the in-plane and out-of-plane displacements of each wall.



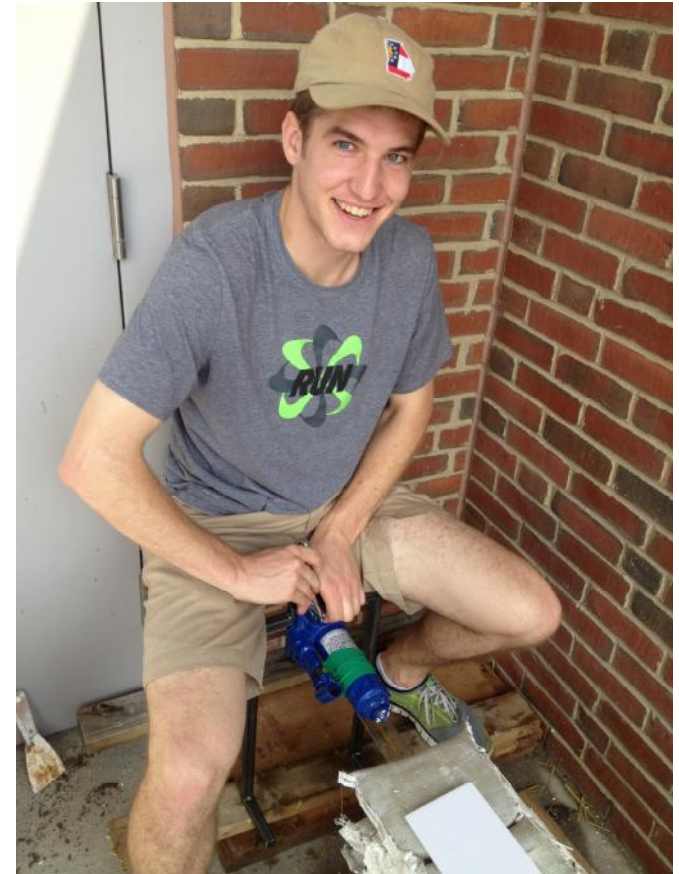
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# ENGS 71 GROUP PROJECT

- § In order to compare our experimental results with theoretical predictions, we needed to experimentally determine the Young's Modulus of an earth-bag.
- § We used the Instron to compress a single earth-bag. The Force vs. Extension data was used to create a Stress – Strain plot.
- § The slope of the linear portion of the graph (Young's modulus) was determined to be approx. 20 MPa.
- § The wall was modeled as a simply supported beam with maximum deflection at the center of the wall.
- § Our test results were **inconclusive** and our experimental readings did not match our predictions.

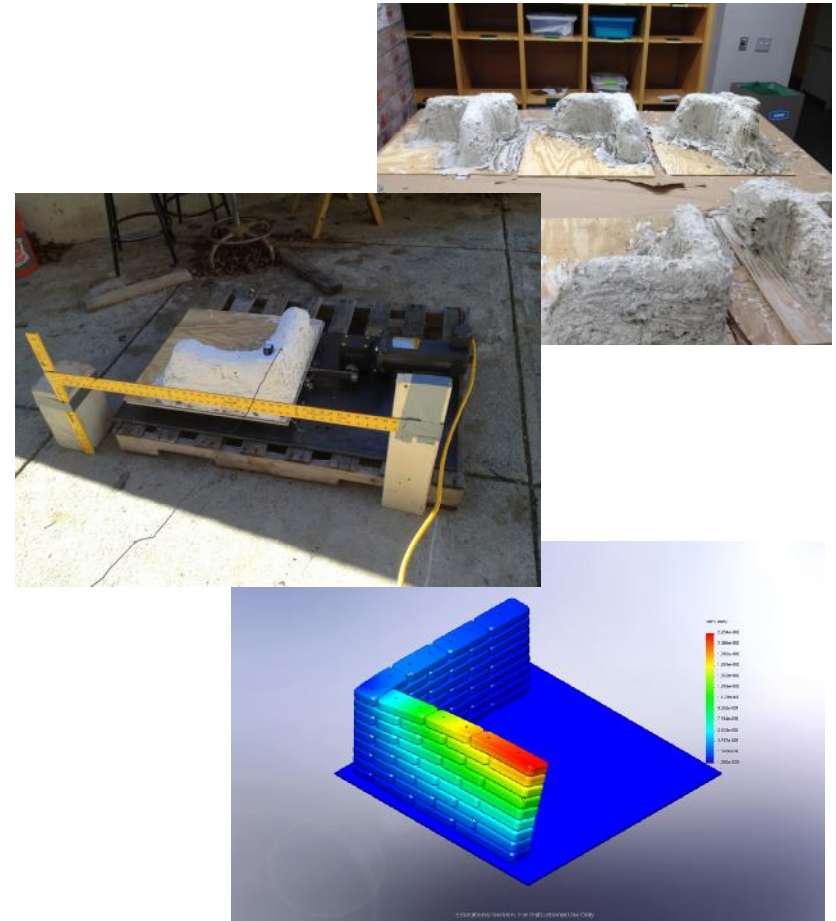
**WE NEEDED A MORE ACCURATE APPROACH TO TESTING.**



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# PROJECT GOALS

- Build scaled down models of earth-bag corner walls (regular corners, piers and buttresses).
- Test effectiveness of reinforcing corners using a **shake table**.
- Measure real-time deformations of the walls up to failure and scale back up to determine the deformations of real-life structures. Compare max. displacements with theoretical predictions.
- Develop a CAD model of the walls and use SolidWorks Simulation to determine the max. deflections in each type of corner wall.



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# DESIGN METHODOLOGY

- **Ian Herrick '13** built a shake table for Professor May during the Summer 2012 term. The weight limit of the table determined the scale factor of the corner walls.
- Max. Weight allowed was approx. 100 lbs. The appropriate scale factor was 1/6<sup>th</sup> of the real-life wall.
- I built two walls of each corner type. The wall dimensions were 20 x 20 square inches.
- Each bag was 2.5 inches wide, and the length of the bags varied from 5 inches to 15 inches depending on location and type of corner wall.
- 1/8<sup>th</sup> in. thick 24 x 24 in<sup>2</sup> sheets of plywood were used as the base of the walls.
- The total weight of each wall was calculated to be between 50 – 60 lbs.



## **Shake Table Specifications:**

Total travel: 1.4 inches

Table Dimensions: 24 in. by 24 in.

Total Weight: 325 lbs.

Weight Limit for Testing: 100 lbs.

Frequency Range: 0 Hz to 5.8 Hz

Motor: Baldor 0.5 HP, 90 V, DC Gear

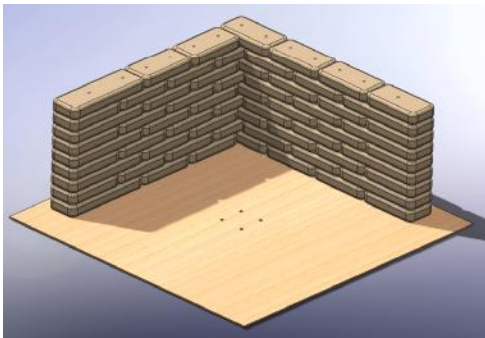
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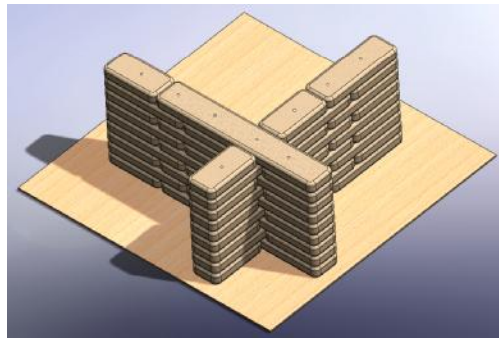
# DESIGN METHODOLOGY

The Curious Case of scaled down models!

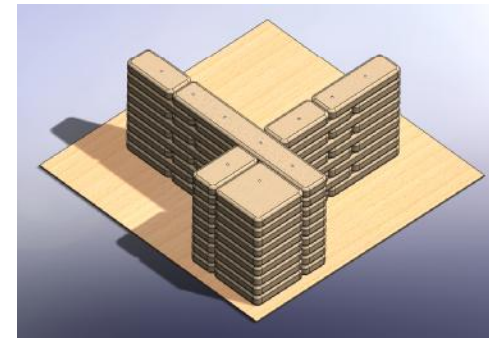
- The earth-bags were not the only parts of the model that needed scaling.
- Barbed wire and rebar also needed to be scaled down.
- Required barbed wire diameter = 0.06 in.
- Required rebar diameter = 0.2 in.



Regular Corner



Pier



Buttress

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# CORNER WALLS CONSTRUCTION

## Getting the right soil (Ball Drop Test)

- Wet soil just enough to hold together. Form into 4 cm (1.5 inch) diameter balls.
- Drop them from 1.5 m (5') height onto a hard surface. If they flatten only slightly the soil has too much clay and needs more sand.
- If the balls develop cracks (break into 4 or 5 pieces), the soil is good for earth-bags.
- If the balls shatter, the soil does not have enough clay or moisture.



## Plastering (2-layer Process)

- Layer 1: Mixture of Soil and Straw
  - Ø Provides a rough, flat surface on which the plaster can stick and harden easily.
- Layer 2: Plaster of Paris
  - Ø Used a 2:1 ratio of water to plaster.



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# CORNER WALLS CONSTRUCTION

Shouldn't building a smaller model be easier?

- Answer: Yes and No.
- Less labor-intensive, but more time-intensive.
- The polypropylene woven bags needed to be cut down to size and then stitched to form a bag. (Time sink. The sewing machine kept breaking down!)
- I had to use a tablespoon to fill each bag, and the handle end of a hammer for tamping the soil firmly inside the bags. (Another time sink!)
- The 0.06 in. diameter wire had to be cut into barbed pieces using a wire cutter. (Yet another time sink!)
- Took me 6 weeks to build 6 walls!



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# SHAKE TABLE TESTING

- Testing took place on November 20<sup>th</sup>, 2012.
- Conducted by Awais Malik and Ian Herrick.
- The wooden base of the walls were bolted to the shake table.
- One side of the corner walls was perpendicular to the direction of motion. The other side was parallel.
- Two 3-axis accelerometers were fixed on to the top of the walls.
- A high-speed camera was also set up to measure displacement of a fiducial point.
- However, memory storage became an issue. The camera could take continuous snapshots for only 1 minute. Our experimental requirement was at least 5 minutes, so we could not use the camera.
- Initial shaking frequency was 1.5 Hz with an incremental increase of 0.5 Hz after every minute until failure.



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# OBSERVATIONS & RESULTS

## Observations:

- Both the regular corners failed at a shaking frequency of 4.0 Hz. Failure occurred at the corners.
- The piers were slightly more resistant than regular corners. One wall failed at 4.5 Hz. The other at 4 Hz. Failure did NOT occur at the corner.
- Both buttresses failed at 4.0 Hz. The wall remained intact as the outer column fell off.

Regular Corner



Pier



Buttress

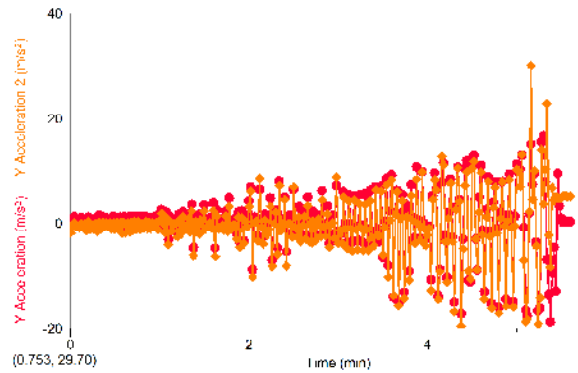


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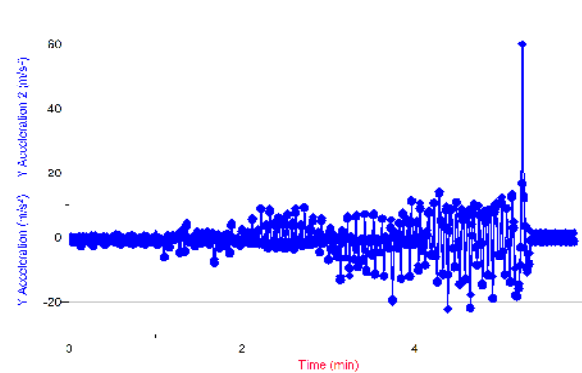
# ACCELEROMETER RESULTS

## Acceleration vs. Time Plots

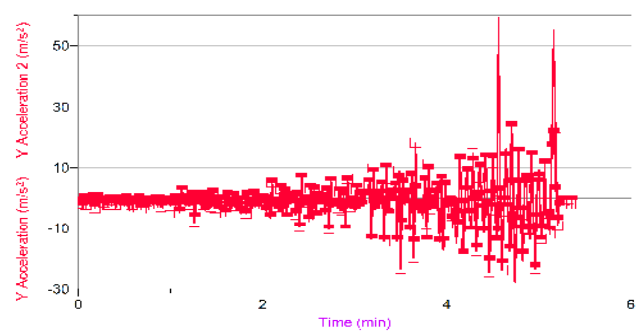
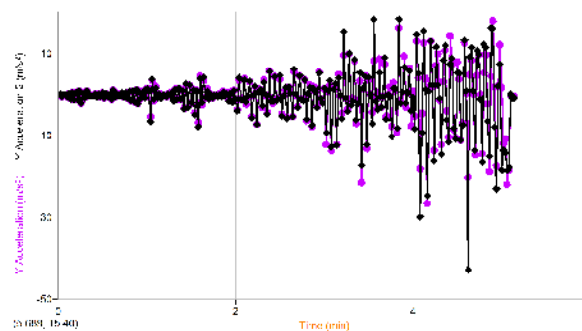
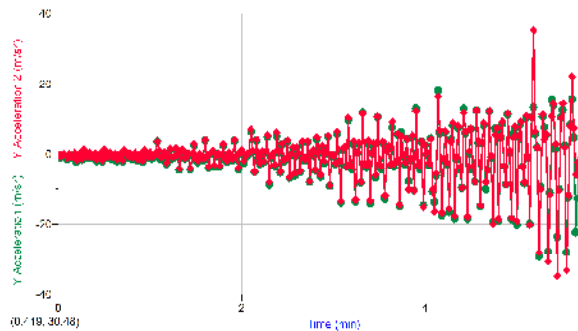
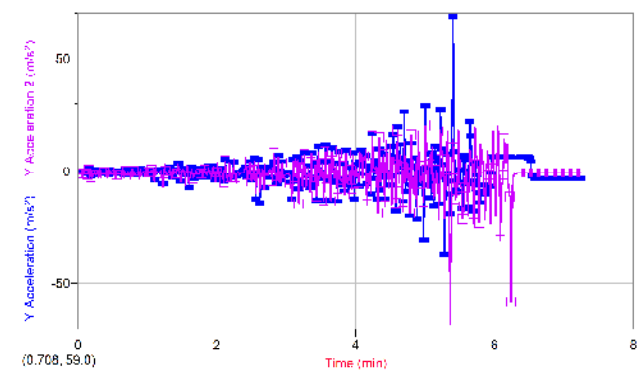
Regular Walls



Piers



Buttresses

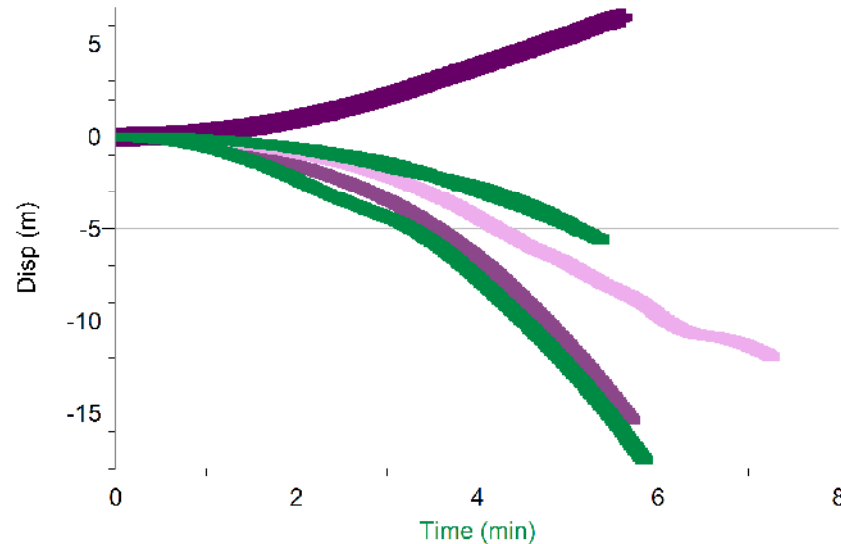


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# ANALYSIS OF TEST RESULTS

## Displacement vs. Time Plots



**This data needs to be corrected!**

Houston, we have a problem!

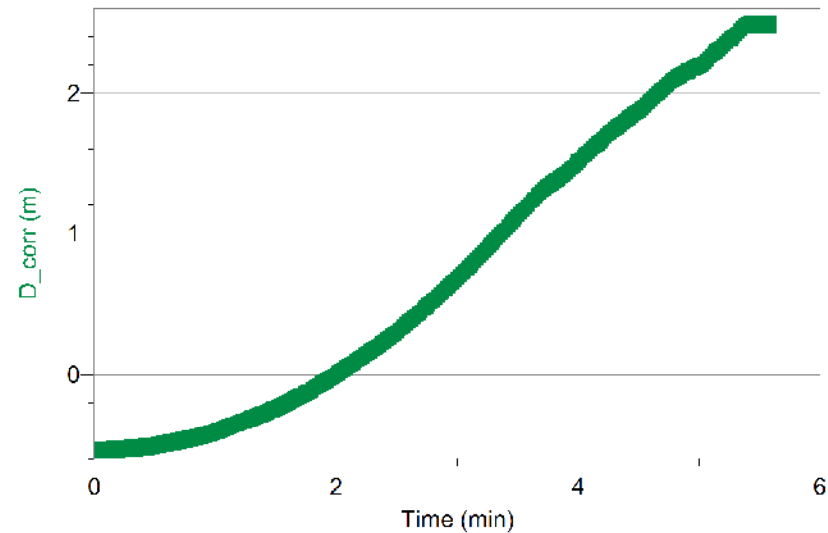
- The displacement graphs should be sinusoidal, not exponential!
- Problem lies in the use of accelerometers. Very difficult to extract displacement from acceleration.
- The slightest of offsets will make the accelerometer think that the wall is moving continuously in a certain direction instead of oscillating back and forth.

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# ANALYSIS OF TEST RESULTS

## Correction Attempt

- Subtracted the mean displacement from each individual displacement reading.
- Improves the readings slightly, but still no where near predicted results.
- Developing an algorithm to produce an accurate set of displacement data is an Honors thesis project in itself.

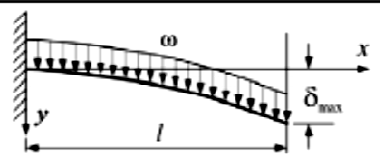


Regular Wall 1

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# THEORETICAL PREDICTION

- **Model the side of the wall perpendicular to the direction of motion as a Cantilever beam. Assume the wall parallel to the direction of motion acts as a fixed support.**

3. Cantilever Beam - Uniformly distributed load $\omega$ (N/m)			
	$\theta = \frac{\omega l^3}{6EI}$	$y = \frac{\omega x^2}{24EI} (x^2 + 6l^2 - 4lx)$	$\delta_{\max} = \frac{\omega l^4}{8EI}$
	Slope at free end	Deflection at length, x, away from fixed end	Max. Deflection at free end

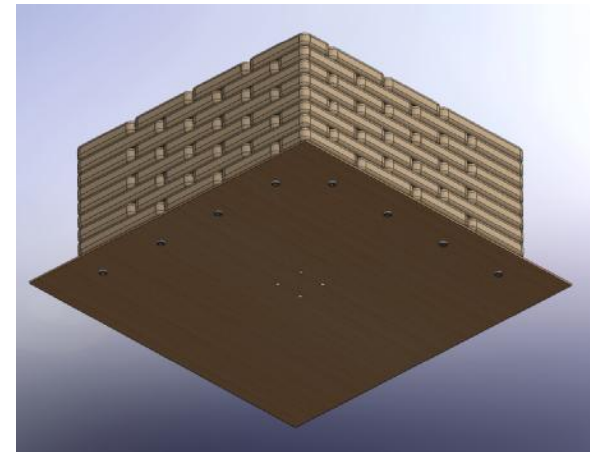
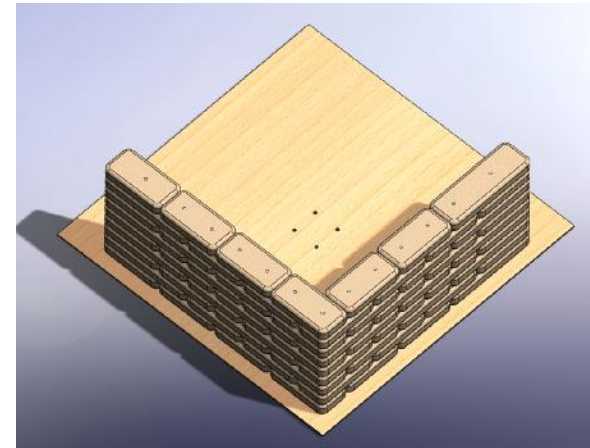
- **Force = mass x acceleration = 255.0 N; mass = 50 lbs. / 2 = 25 lbs. = 11.34 kg; acceleration =  $4\pi^2 f^2 x = 22.49 \text{ m/s}^2$**   
(Frequency,  $f = 4.0 \text{ Hz}$ ; Distance,  $x = 1.4 \text{ inches} = 0.0356 \text{ m}$ )
- Distributed load,  $\omega = \frac{\text{Force}}{\text{length}} = \frac{255 \text{ N}}{0.445 \text{ m}} = 573.0 \frac{\text{N}}{\text{m}}$
- Young's Modulus,  $E = 2.0 \times 10^7 \text{ Pa}$
- Moment of Inertia,  $I = \frac{bh^3}{12} = \frac{2.5 \times 9^3}{12} = 151.875 \text{ inch}^4 = 6.3215 \times 10^{-5} \text{ m}^4$
- Maximum deflection,  $\delta_{MAX} = \frac{\omega l^4}{8EI} = 2.22 \text{ mm}$

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# DEVELOPMENT OF CAD MODELS

- SolidWorks: the final frontier! 3D Mechanical CAD modeling software with powerful simulation tools.
- The earth-bags were modeled as rectangular bars with fillets on each edge.
- A custom material was created and applied to the earth-bag extrudes.
- Used cylindrical pins as barbs that fitted into extruded cuts in between each earth-bag row.
- However, SolidWorks detected interferences as a result of the Pattern feature on the cuts.
- If SolidWorks detects any interference, it cannot create a mesh for simulation.
- As an adjustment, I removed the barbs and their respective extruded cuts, and replaced them with rebar.
- Now a very fine mesh was formed, and the finite element analysis (FEA simulation) could run.



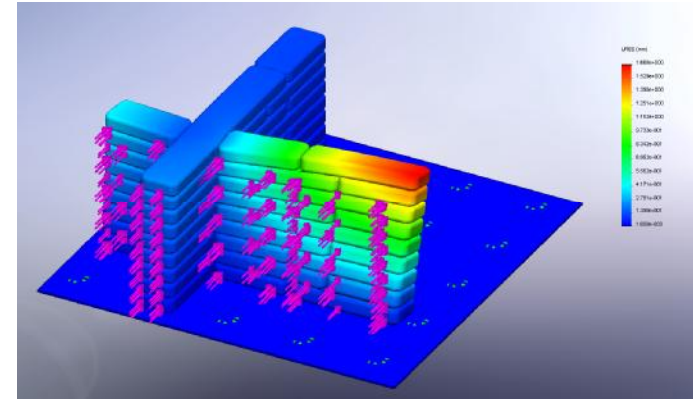
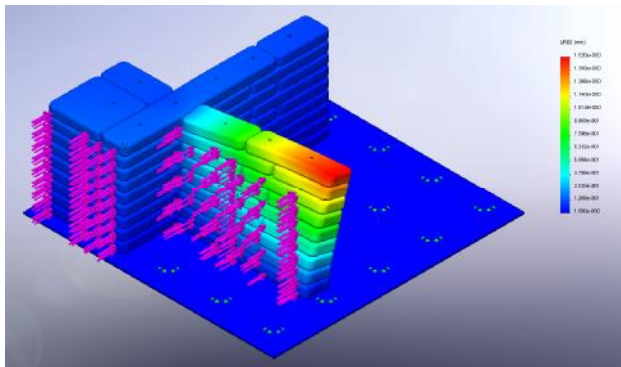
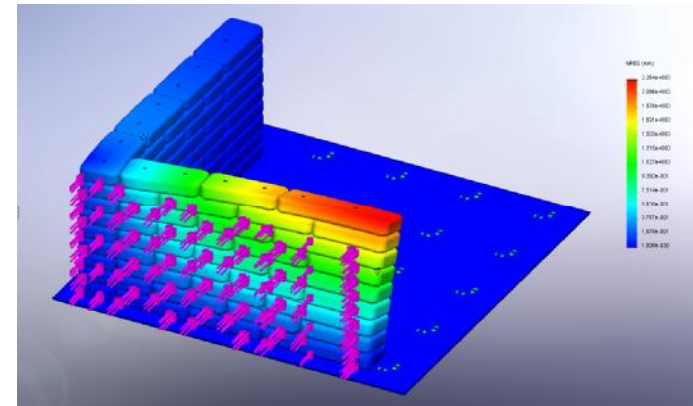
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# SOLIDWORKS SIMULATIONS

## Maximum Deflections:

- Regular Corner – 2.254 mm  
Real-life wall – 13.52 mm
- Piers – 1.668 mm (26% decrease)  
Real-life wall – 10.00 mm
- Buttresses – 1.520 mm (33% decrease)  
Real-life wall – 9.12 mm

SolidWorks Simulations and Theoretical Predictions match up very well! (1.5% error)



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# CONCLUSIONS

- Corner reinforcements increase the wall's resistance to vibration (higher failure frequency and lower max. deflection), and are worth the added investment.
- The SolidWorks models and the cantilever beam model should be accurate predictors of the behavior of earth-bag systems under earthquake conditions.



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# FUTURE RECOMMENDATIONS

- Larger scale testing to obtain more accurate failure modes of real-life structures.
- Use **Motion Detectors** that measure distance directly in place of accelerometers.
- Reinforce the stub ends of the walls with meshing.
- Ensure earth-bag edges do **NOT line up**.
- Do such projects as part of a **GROUP!**



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# ACKNOWLEDGEMENTS

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- Professor Vicki May
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- Ian Herrick
- David Connolly
- Steven Jin



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# THANK YOU!



Thayer School of Engineering at Dartmouth  
14 Engineering Drive  
Hanover, NH 03755

[engineering.dartmouth.edu](http://engineering.dartmouth.edu)

## Questions? Comments?

